

# Discussion of "Case Study of Prototype Hydraulic Jump on Slope: Air Entrainment and Free-Surface Measurement" by Zhongtian Bai, Ruidi Bai, Rongcai Tang, Hang Wang, and Shanjun Liu

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The original article presented a challenging and interesting case study of hydraulic jump in a real-world application and discussed a number of very relevant practical considerations. In this discussion, it is argued that some air-water results showed differences from traditional air-water flow results in the laboratory for a number of reasons, including different inflow and boundary conditions. In turn, one needs to be careful with some definitive conclusions, especially based on one flow condition with a Reynolds number close to some of the largest laboratory studies to date.

The authors did well to discuss the use of phase-detection needle probes in the field and the associated practical considerations. During field studies, needle probe sensors may be damaged by quartz particles and small debris, as previously discussed at the Aviemore Dam spillway (Keller 1972; Cain 1978) and reported in several laboratory studies conducted with high velocities (Low 1986; Cummings 1996; Gonzalez 2005). Furthermore, the water quality, including salinity and presence of surfactants, may adversely affect the signal outputs of phase-detection electrical probes (Timkin et al. 2003; Chanson et al. 2006; Salter et al. 2014), including the air-water flow properties in hydraulic jumps (Reif 1978; Pothof et al. 2013). Importantly, the single-threshold technique applied by the authors might not be the best phasediscrimination method in the presence of adverse water quality effects (Jones and Delhaye 1976; Chanson et al. 2002). A doublethreshold technique, a differentiation technique, or a combination of signal thresholds and gradient threshold could be more suitable, although some validation would be required. For example, a combination of signal thresholds and gradient threshold might be performed successfully for the probe signal shown in Fig. 2(a) of the original article. Simply, "the signal processing [of phase-detection needle probes] is not trivial, even in simple steady one-directional flows" (Chanson 2020).

The case study was conducted in a B-jump, i.e., a hydraulic jump down a sloping chute immediately downstream of a sluice gate. Such jumps are also common in some standard culverts with drop inlets and in minimum-energy-loss culvert inlets operating under less than design flow conditions (Fig. 1). Several studies showed that both the boundary conditions and inflow conditions significantly affect the air-water flow properties in hydraulic jumps. Resch and Leutheusser (1972) demonstrated some seminal differences in hydraulic jump rollers with identical inflow Froude numbers between partially and fully developed inflow conditions, the former likely corresponding to the case study inflow. Mignot and Cienfuegos (2011) showed that turbulence production was primarily confined to the developing shear layer in the case of partially

developed inflow, with Chanson and Brattberg (2000) arguing that the free-surface velocity immediately upstream of the roller is the most relevant velocity scale. Thandaveswara (1974) and Stoinic et al. (2021) presented air-water data in hydraulic jumps located downstream of a steep chute, showing the significant role of pre-air entrainment. That is, the inflow aeration modified the hydraulic jump roller characteristics, including its length and the air diffusion process. In horizontal channels with partially developed inflow, several studies investigated the effects of large bed roughness (Pagliara et al. 2010; Felder and Chanson 2018; Bahmanpouri et al. 2019). The experimental observations showed a drastic modification of both roller flow patterns and air-water flow properties. Fig. 2 presents a comparison in terms of the vertical distributions of void fraction and dimensionless frequency for three data sets measured for a similar Froude number Fr1 at comparable dimensionless locations  $x/d_1$  from the roller toe (Table 1). That is, the case study (B-jump) data are compared to classical hydraulic jump data (smooth invert) (Chachereau and Chanson 2011) and measurements in a horizontal hydraulic jump with artificial bed roughness (Felder and Chanson 2018). The comparative results showed markedly different trends, aeration levels, and fragmentation rates. Such differences were most likely linked to the different boundary conditions because all the data (in Fig. 2) were collected with similar dual-tip phase-detection probes and characterized by some partially developed inflow (Table 1). Simply, any true comparison of airwater flow properties in hydraulic jumps must be based on data sets with comparable boundary and inflow conditions.

Finally, the authors are correct to remind us of the absence of benchmark data sets for any investigations of scale effects affecting the air-water flow properties in large prototype stilling basin operating at large Reynolds numbers ( $\text{Re} > 10^7$ ). Yet, a recent data set (Estrella et al. 2022) investigated the air-water flow properties in



**Fig. 1.** B-jump in a minimum-energy-loss culvert inlet at Stones Corner on March 30, 2017 (shutter speed = 1/200 s,  $\alpha = 4^{\circ}$ ).



**Fig. 2.** Comparative distributions of void fraction *C* and dimensionless bubble frequency  $f \cdot d_1/V_1$  in hydraulic jumps with comparable inflow Froude number Fr<sub>1</sub> at similar dimensionless locations  $x/d_1$  (Table 1): (a) void fraction; and (b) bubble frequency.

**Table 1.** Comparative studies of air entrainment in hydraulic jumps with comparable inflow Froude number  $Fr_1$  at similar dimensionless locations  $x/d_1$ 

Reference	Invert slope $\alpha$	Invert conditions	$d_1$	Fr <sub>1</sub>	Re	$x/d_1$	Comment
Case study	15.5	Sloping concrete ( $\alpha = 15.5^{\circ}$ )	0.145	5.1	$9 \times 10^5$	9.3, 13.8	B-jump
Felder and Chanson (2018)	0	Artificial roughness 1 ( $k_s \sim 12 \text{ mm}$ )	0.045	5.5	$1.6 \times 10^{5}$	7.8	Rough invert
Chachereau and Chanson (2011)	0	PVC	0.0395	5.1	$1.3 \times 10^5$	7.6, 15.2	Smooth invert

weak hydraulic jump with Reynolds number between  $0.08 \times 10^5$ and  $3 \times 10^5$ . At the largest Reynolds number, the results did show a sizable number of large bubbles, in amounts comparable to the case study. The finding was further consistent with other laboratory studies at higher Froude numbers (Chanson 2007; Montano and Felder 2020). In the present case study, the results might not be truly Froude-similar to past laboratory studies because of the marked differences in inflow and boundary conditions. Maybe the key outcomes are (1) a need for field measurements of air-water flow in large prototypes; and (2) the necessity for detailed air-water flow measurements in B-jump, to complement the present case study.

## **Data Availability Statement**

Some or all data, models, or code that support the findings of this study are available from the corresponding author upon reasonable request.

## Notation

The following symbols are used in this paper:

- $d_1 = \text{inflow depth (m)};$
- $Fr_1 = inflow$  Froude number;
- Re = Reynolds number;
- *x* = longitudinal distance (m) from the roller toe positive downstream;

y = vertical distance (m) measured normal to the invert; and  $\alpha =$  angle between longitudinal invert slope and horizontal.

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